Drainage Narrative Keating Residence Cliff Road, Tuxedo Park, NY Rev. 7/17/14 (to match plan date, no change in design)

Introduction

The intent of this report is to analyze existing versus proposed storm water run-off characteristics for proposed development of the above site, on the west side of Cliff Road, in the Village of Tuxedo Park. We utilized "Hydraflow Hydrographs" software and the modified rational method for determining times of concentration, peak flows and detention system routings.

Existing Conditions

The property in question has a total area of 2.03 acres. It currently is vacant. The property contains large rock outcrops and is steeply sloped. The property's run-off is divided along a ridge, with 1.55 acres flowing west and 0.48 acres flowing east. The existing drainage area map is attached. The lot once contained a partially constructed dwelling that was demolished due to structural issues. Part of that development included at least four (4) seepage pits for roof leader run-off. The current owner has investigated the condition and capacity of these pits and intends to utilize them for proposed storm water detention on site.

Proposed Conditions

The applicant proposes to construct a new single family dwelling, driveway, pool and patio. The intent of this report is to match the existing peak flows, both easterly and westerly, as closely as possible post-development. Proposed drainage area maps are attached.

The flow to the west will include the proposed driveway and the northerly side yard of the proposed dwelling. The driveway run-off will be collected by a trench drain and piped to the existing seepage pit near the northerly property line. This seepage pit will overflow to a level spreader that will distribute any overflow and allow it to continue flowing in a westerly direction. We are decreasing the overall drainage area to the west so that proposed peak flow rates will be reduced without the need for any storm water detention. As above, the existing seepage pit will be utilized to further attenuate flows, but is not necessary due to the overall reduction in drainage area.

The flow to east will be reduced by utilizing the existing three (3) seepage pits in front of the proposed dwelling. All proposed roof leaders and patio drainage will be piped to the existing seepage pits. The system to the west will be slightly reduced by decreasing the tributary area to the west. The modified rational method hydrographs showing the existing peak flow rate to the east versus proposed flow rates for varying storm durations are attached to this report. The maximum storage of 89 cf (666 gallons) required occurs during the 25-year storm, when Tc=Td=10 minutes. The existing seepage pits have significantly more volume available, and will provide more than adequate storage to decrease peak flows from the site.

The easterly system will be a closed system, with overflow tee connectors on the proposed roof leaders set above the lowest patio perimeter drain elevation of 700.00. In this manner, the system can overflow by backing up flow through the perimeter drain, spreading the flow over the entire length of the house and patio.

Conclusions

Proposed flow to the west will be decreased by a decrease in the overall drainage area. Flow from the driveway will be directed to the existing northerly seepage pit, and overflow via a level spreader which will further mitigate peak flows (even though storage to the west is not required).

Proposed flow to the east will be decreased by utilizing the storage in the three (3) existing seepage pits near the front of the property. Based on the modified rational method, our existing storage is almost ten (10) times what is required. This system will overflow via the proposed pool/patio perimeter drain, spreading the flow over a large area so as not create any concentrated flows.

Thomas Skrable, P.E.

NXSPE #)075377

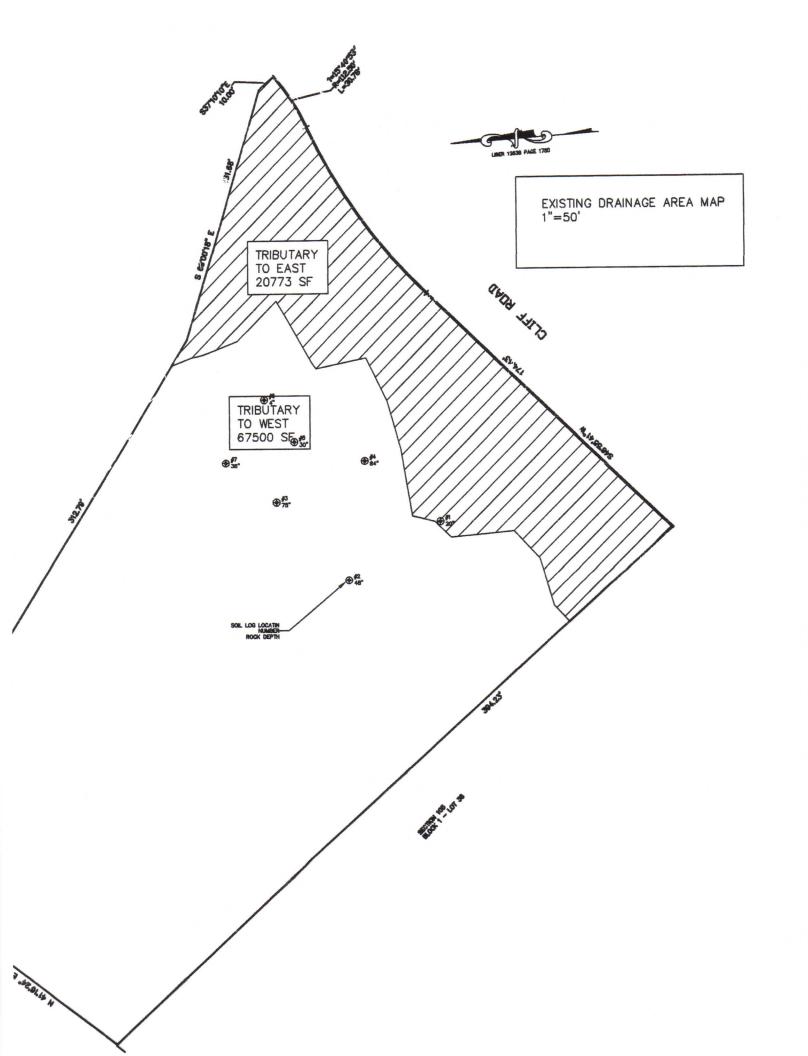
DRAING - CLIFF ROAD SECTION 106 - BLOCK 1 - LOT 75.2

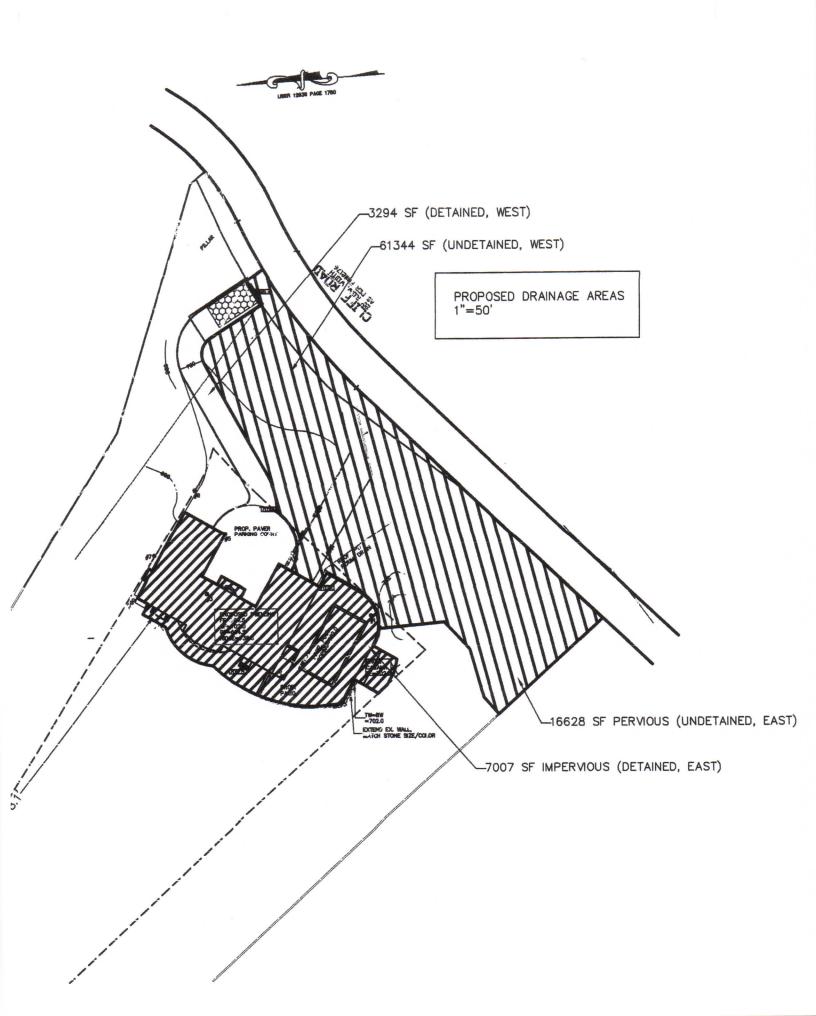
VILLAGE OF TUXEDO PARK - TOWN OF TUXEDO

DATE 1/13/14

REASE TO THE WEST,	ES AND VOLUMES DEC	SE AREA, ALL STORM RAT	DANIARY DRAINAC	2. DUE TO DECREASE IN T
				1. MAX. STORAGE REQUIR
				NOTES:
	and the second s			
A/N	29.0	3.42	5.32	AA3Y 001
A/N	64.0	2.65	61.4	AA3Y 82
A/N	14.0	2.28	3.55	AA3Y 0ト
A/N	82.0	₽ 9"l	2.40	2 YEAR
CE	CES	CES	CES	MAOTS
MAX. STORAGE REQ'D	PROP. DET. WEST	PROP. UNDET. WEST	EX. WEST	
A/N	1.23	1.39	2.31	AA3Y 001
68	Z6 [°] 0	01.1	£8.1	S5 YEAR
84	68.0	≯ 6.0	99.1	AA3Y 0r
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CE	CES	CES	CES	MAOTS
MAX. STORAGE REQ'D		PROP. UNDET. EAST	EX. EAST	

STORAGE REQUIRED.

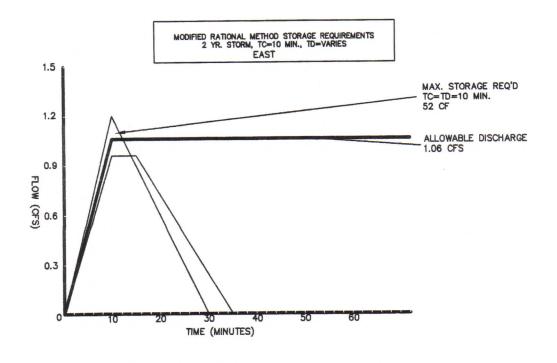


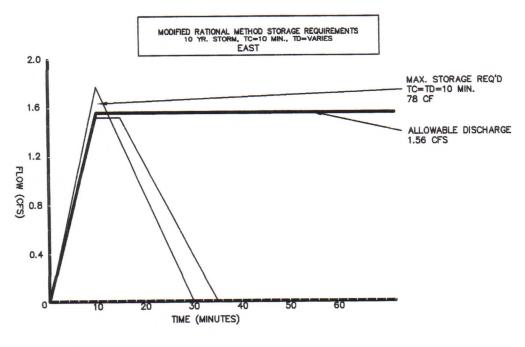


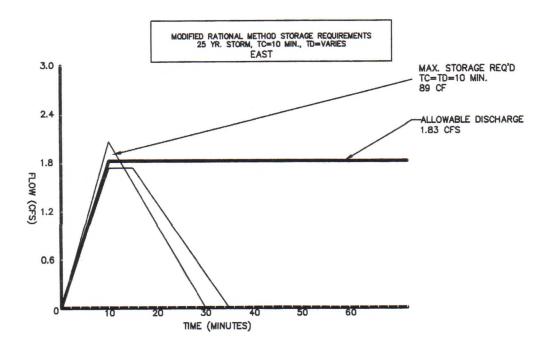
DRAINAGE AREA BREAKDOWN **KEATING - CLIFF ROAD SECTION 106 - BLOCK 1 - LOT 75.2** VILLAGE OF TUXEDO PARK - TOWN OF TUXEDO **DATE 1/13/14**

LOT AREA =	88273	SF	2.03	ACRES
LUI ANLA -	002/0	U 1	4.00	MOI LEO

EXISTING DRAINAGE AREAS:				
1. AREA FLOWING TO THE EAST				
	SF	ACRES	C VALUE	
ROCK OUTCROP	5,000	0.11	0.99	
PERVIOUS, WOODS	15,773	0.36	0.50	
TOTAL AREA	20,773	0.48	0.62	
2. AREA FLOWING TO THE WEST				
	<u>SF</u>	<u>ACRES</u>	C VALUE	
ROCK OUTCROP	1,000	0.02	0.99	
PERVIOUS, WOODS	66,500	1.53	0.50	
TOTAL AREA	67,500	1.55	0.51	
DDODOCED DDAINAGE ADEAS.				
PROPOSED DRAINAGE AREAS:				
1. AREA UNDETAINED TO WEST				
1. AKLA GNOLTANLED TO WEGT	SF	ACRES	C VALUE	
ROCK OUTCROP	1,000	0.02	0.99	
PERVIOUS. LAWN	60,344	1.39	0.35	
TOTAL AREA	61,344	1.41	0.36	
2. AREA DETAINED TO WEST				
	SF	ACRES	C VALUE	
IMPERVIOUS	3,294	0.08	0.99	
PERVIOUS, LAWN	-	-	-	
TOTAL AREA	3,294	0.08	0.99	
3. AREA UNDETAINED TO EAST	0.5	40000	0.1/41.115	
DOCK OUTCOOD	<u>SF</u>	<u>ACRES</u> 0.07	C VALUE 0.99	
ROCK OUTCROP PERVIOUS, LAWN	3,000 13,628	0.07	0.99	
TOTAL AREA	16,628	0.31	0.33	
TOTAL AINLA	10,020	0.00	0.47	
4. AREA DETAINED TO EAST				
	SF	ACRES	C VALUE	
ROCK OUTCROP	7,007	0.16	0.99	
PERVIOUS	-	-	-	
TOTAL AREA	7,007	0.16	0.99	







PROPOSED SEEPAGE PIT STORAGE CALCULATIONS SINGLE NORTHERLY PIT KEATING, CLIFF ROAD VILLAGE OF TUXEDO PARK

STORAGE REQUIRED

STORAGE VOLUME REQUIRED	0 GALLONS	
SEEPAGE PIT STORAGE VOLUME		
NUMBER OF PITS:	1 PITS	
INSIDE DIAMETER	6 FT	
INSIDE HEIGHT	2.5 FT**	
TOTAL HEIGHT	3.42 FT	
VOLUME INSIDE PIT	70.69 CF	
	528.73 GALLONS	
STONE STORAGE VOLUME		
LENGTH	6.5 FT	
WIDTH	6.5 FT	
DEPTH BELOW PITS	0 FT	
OUTSIDE DIAM. OF PIT	6.5 FT	
STONE BELOW PITS	0 CF	
	0 GALLONS	
STONE OUTSIDE PITS (BELOW INVERT)	9.07 CF	
	67.82 GALLONS	
TOTAL STONE	67.82 GALLONS	
PITS AND STONE CAPACITY:		
PITS	528.73	
STONE	67.82	
STORAGE VOLUME PROVIDED	596.55 GALLONS	

ASSUMED STONE VOIDS = 40%

^{** -} HEIGHT REDUCED DUE TO 4" STANDING WATER IN PIT

PROPOSED SEEPAGE PIT STORAGE CALCULATIONS THREE (3) SOUTHERLY PITS KEATING, CLIFF ROAD VILLAGE OF TUXEDO PARK

STORAGE REQUIRED

STORAGE VOLUME REQUIRED	666 GALLONS (89 CF)		
SEEPAGE PIT STORAGE VOLUME			
NUMBER OF PITS:	3 PITS		
INSIDE DIAMETER	6 FT		
INSIDE HEIGHT	5.23 FT**		
TOTAL HEIGHT	6.25 FT		
VOLUME INSIDE PIT	443.62 CF		
	3318.31 GALLONS		
STONE STORAGE VOLUME			
LENGTH	22 FT		
WIDTH	10 FT		
DEPTH BELOW PITS	2 FT		
OUTSIDE DIAM. OF PIT	6.5 FT		
STONE BELOW PITS	176 CF		
	1316.48 GALLONS		
STONE OUTSIDE PITS	251.98 CF		
	1884.83 GALLONS		
TOTAL STONE	3201.31 GALLONS		
PITS AND STONE CAPACITY:			
PITS	3318.31		
STONE	3201.31		
STORAGE VOLUME PROVIDED	6519.62 GALLONS		

ASSUMED STONE VOIDS = 40%

^{** -} HEIGHT REDUCED DUE TO 0.44' VARIATION IN TANK ELEVATIONS

Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:10 AM

Hyd. No. 1

EXISTING TO EAST

Hydrograph type = Rational
Storm frequency = 2 yrs
Drainage area = 0.480 ac
Intensity = 3.556 in/hr
IDF Curve = rockland.IDF

5

Hyd No. 1

0

10

Peak discharge = 1.06 cfs
Time interval = 1 min
Runoff coeff. = 0.62
Tc by User = 10.00 min
Asc/Rec limb fact = 1/2

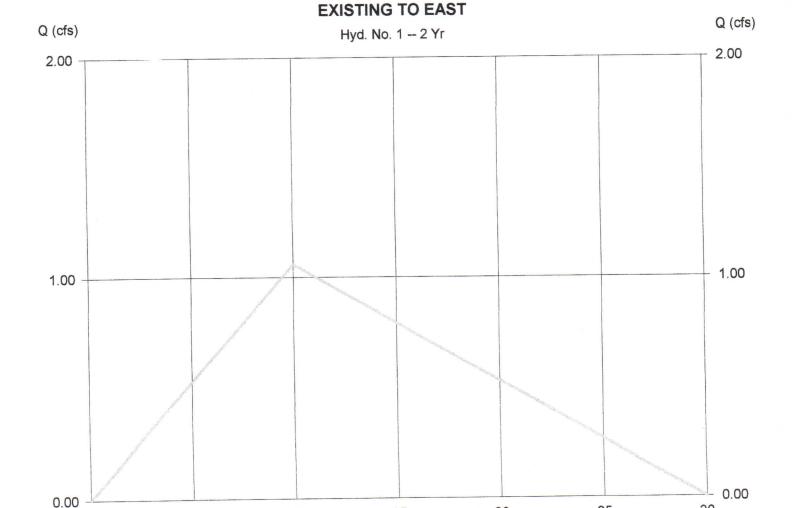
Hydrograph Volume = 952 cuft

30

Time (min)

25

20



15

Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

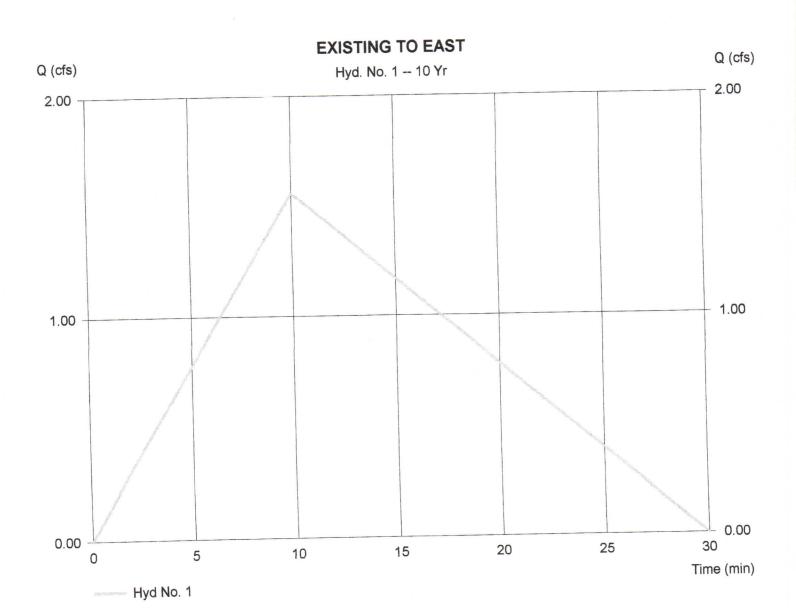
Hyd. No. 1

EXISTING TO EAST

Hydrograph type = Rational
Storm frequency = 10 yrs
Drainage area = 0.480 ac
Intensity = 5.240 in/hr
IDF Curve = rockland.IDF

Peak discharge = 1.56 cfs
Time interval = 1 min
Runoff coeff. = 0.62
Tc by User = 10.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 1,403 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

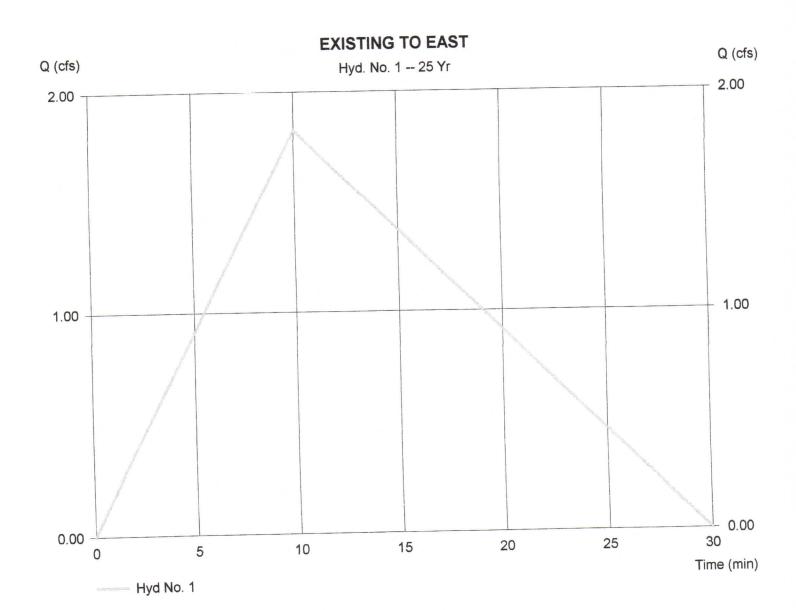
Hyd. No. 1

EXISTING TO EAST

Hydrograph type = Rational
Storm frequency = 25 yrs
Drainage area = 0.480 ac
Intensity = 6.135 in/hr
IDF Curve = rockland.IDF

Peak discharge = 1.83 cfs
Time interval = 1 min
Runoff coeff. = 0.62
Tc by User = 10.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 1,643 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

Hyd. No. 1

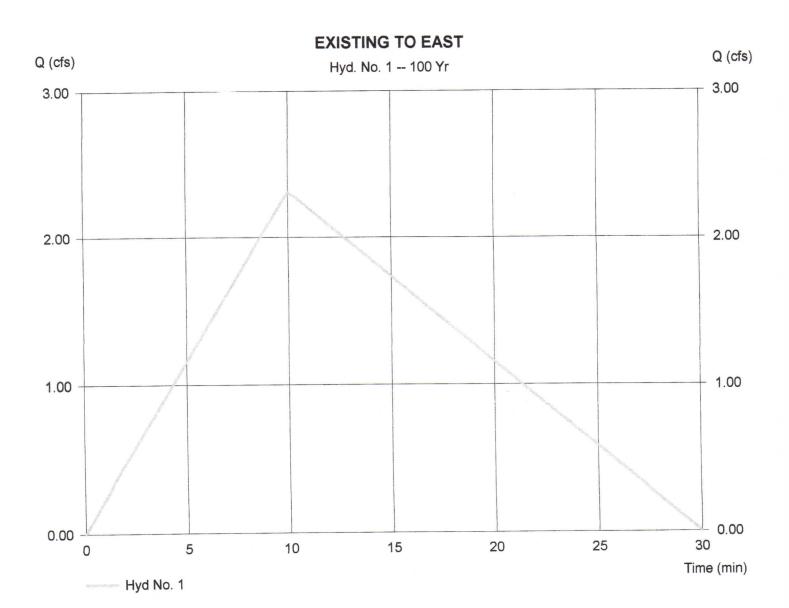
EXISTING TO EAST

Hydrograph type = Rational
Storm frequency = 100 yrs
Drainage area = 0.480 ac
Intensity = 7.774 in/hr
IDF Curve = rockland.IDF

Peak discharge = 2.31 cfs
Time interval = 1 min
Runoff coeff. = 0.62
Tc by User = 10.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 2,082 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

Hyd. No. 3

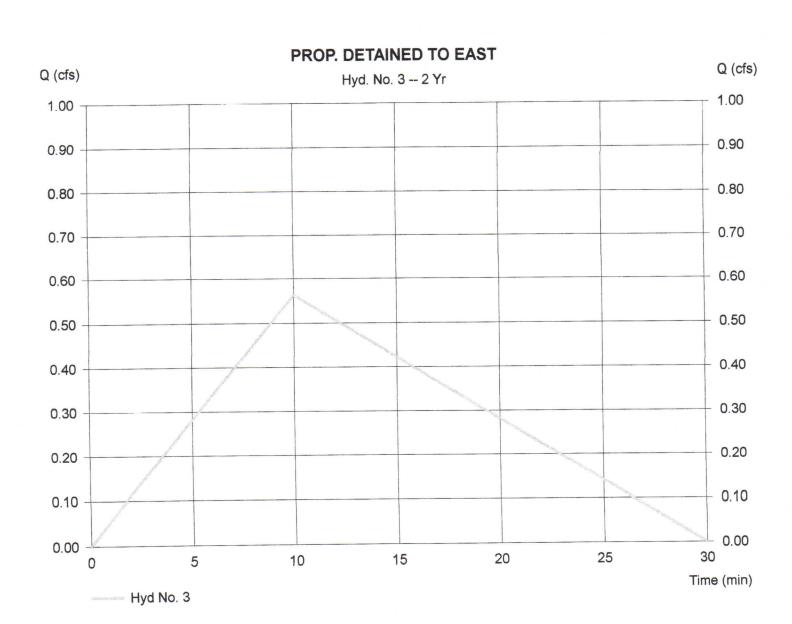
PROP. DETAINED TO EAST

Hydrograph type = Rational
Storm frequency = 2 yrs
Drainage area = 0.160 ac
Intensity = 3.556 in/hr

Intensity = 3.556 in/hr IDF Curve = rockland.IDF Peak discharge = 0.56 cfs Time interval = 1 min Runoff coeff. = 0.99

Tc by User = 10.00 minAsc/Rec limb fact = 1/2

Hydrograph Volume = 507 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

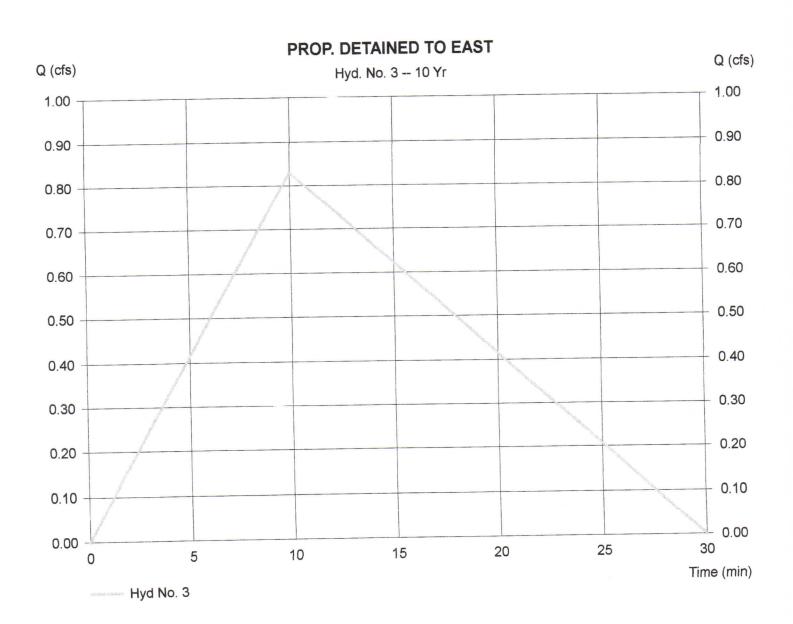
Hyd. No. 3

PROP. DETAINED TO EAST

Hydrograph type = Rational
Storm frequency = 10 yrs
Drainage area = 0.160 ac
Intensity = 5.240 in/hr
IDF Curve = rockland.IDF

Peak discharge = 0.83 cfs
Time interval = 1 min
Runoff coeff. = 0.99
Tc by User = 10.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 747 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:12 AM

Hyd. No. 3

PROP. DETAINED TO EAST

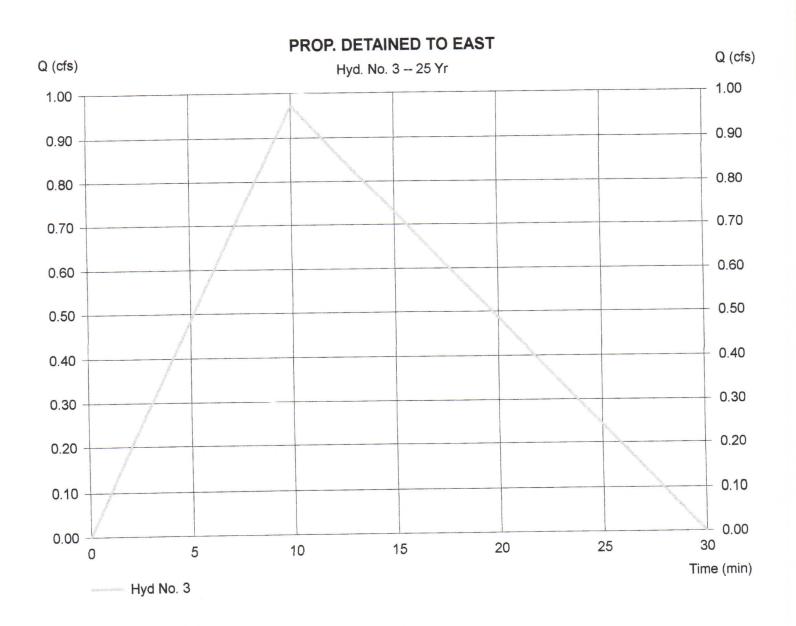
Hydrograph type = Rational
Storm frequency = 25 yrs
Drainage area = 0.160 ac
Intensity = 6.135 in/l

IDF Curve

= 6.135 in/hr = rockland.IDF Peak discharge = 0.97 cfs
Time interval = 1 min
Runoff coeff. = 0.99
Tc by User = 10.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 875 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:12 AM

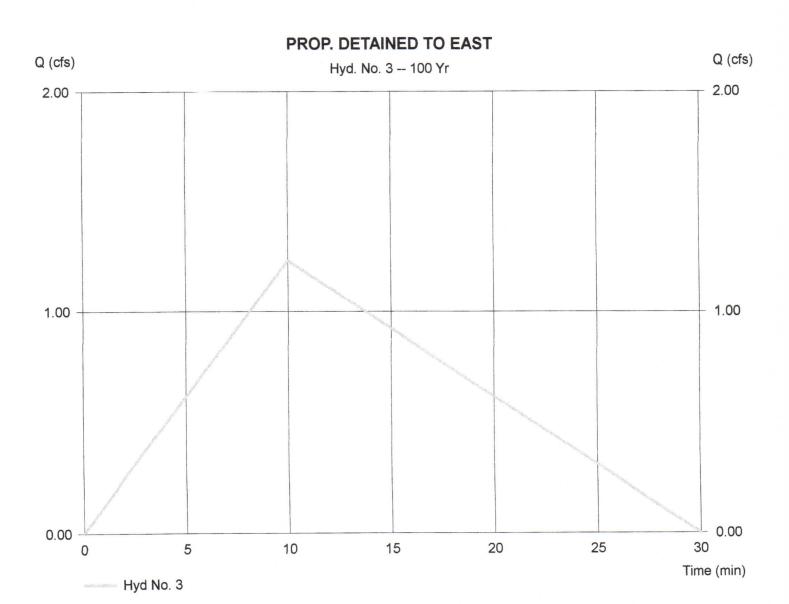
Hyd. No. 3

PROP. DETAINED TO EAST

Hydrograph type = Rational
Storm frequency = 100 yrs
Drainage area = 0.160 ac
Intensity = 7.774 in/hr
IDF Curve = rockland.IDF

Peak discharge = 1.23 cfs
Time interval = 1 min
Runoff coeff. = 0.99
Tc by User = 10.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 1,108 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

Hyd. No. 2

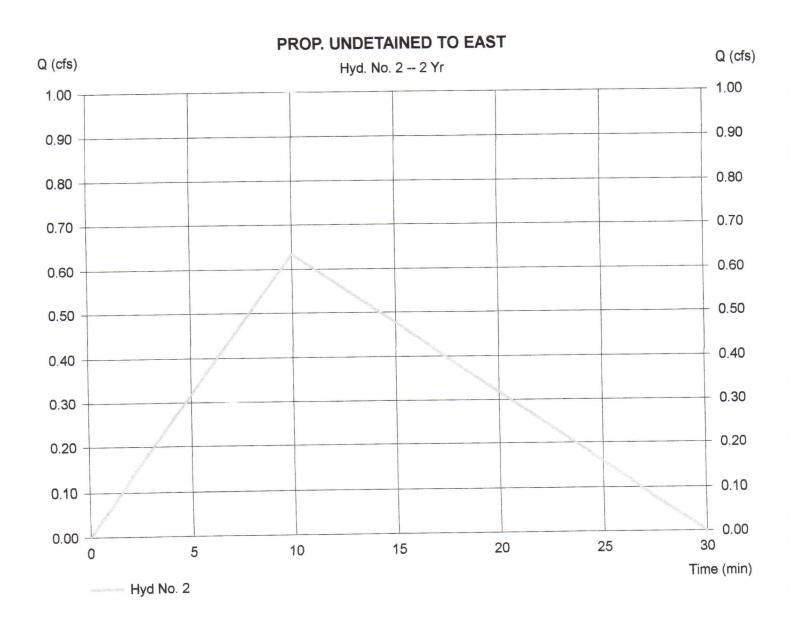
PROP. UNDETAINED TO EAST

Hydrograph type = Rational
Storm frequency = 2 yrs
Drainage area = 0.380 ac
Intensity = 3.556 in/hr
IDF Curve = rockland.IDF

Peak discharge = 0.64 cfs
Time interval = 1 min
Runoff coeff. = 0.47
Tc by User = 10.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 572 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

Hyd. No. 2

PROP. UNDETAINED TO EAST

Hydrograph type = Rational Storm frequency = 10 yrs= 0.380 acDrainage area Intensity

IDF Curve

= 5.240 in/hr = rockland.IDF Peak discharge Time interval

= 0.94 cfs

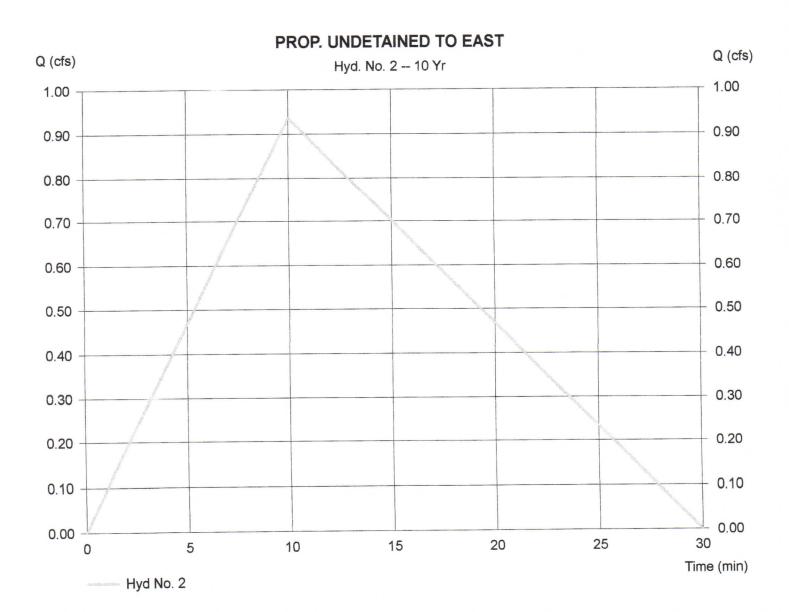
= 1 min Runoff coeff. = 0.47

Tc by User

 $= 10.00 \, \text{min}$

Asc/Rec limb fact = 1/2

Hydrograph Volume = 842 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

= 1.10 cfs

Hyd. No. 2

IDF Curve

0.00

0

5

Hyd No. 2

10

PROP. UNDETAINED TO EAST

Hydrograph type = Rational
Storm frequency = 25 yrs
Drainage area = 0.380 ac
Intensity = 6.135 in/hr

= 25 yrs
= 0.380 ac
= 0.135 in/hr
= rockland.IDF

Time interval
= 1 min
Runoff coeff.
= 0.47
To by User
= 10.00 min
Asc/Rec limb fact
= 1/2

Peak discharge

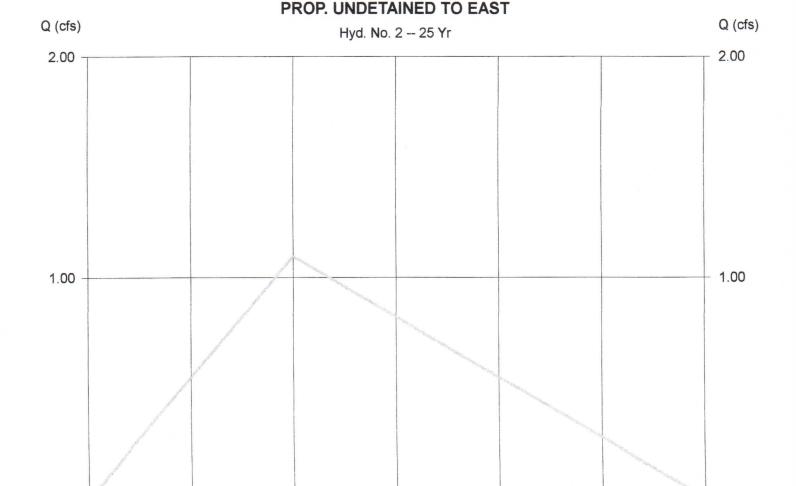
Hydrograph Volume = 986 cuft

0.00

30

Time (min)

25



15

20

Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:11 AM

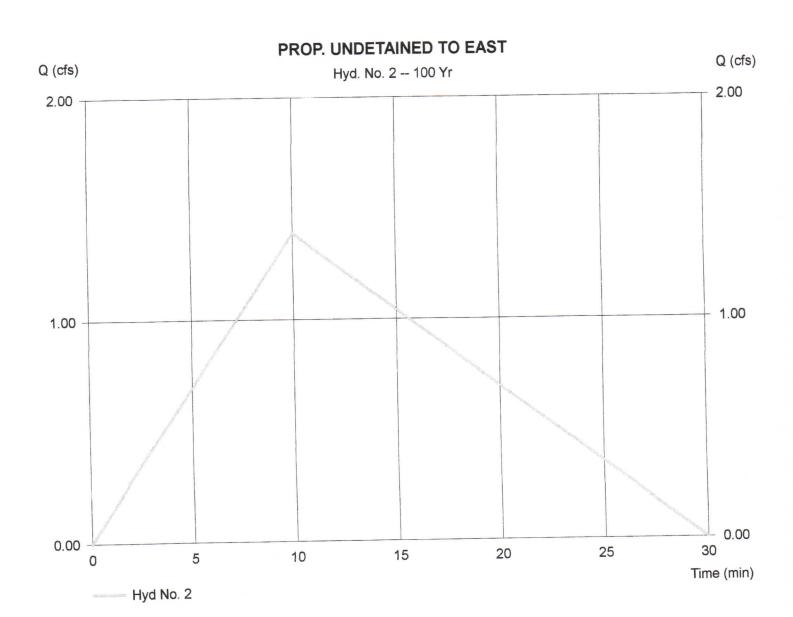
Hyd. No. 2

PROP. UNDETAINED TO EAST

Hydrograph type = Rational
Storm frequency = 100 yrs
Drainage area = 0.380 ac
Intensity = 7.774 in/hr
IDF Curve = rockland.IDF

Peak discharge = 1.39 cfs
Time interval = 1 min
Runoff coeff. = 0.47
Tc by User = 10.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 1,250 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:23 AM

Hyd. No. 4

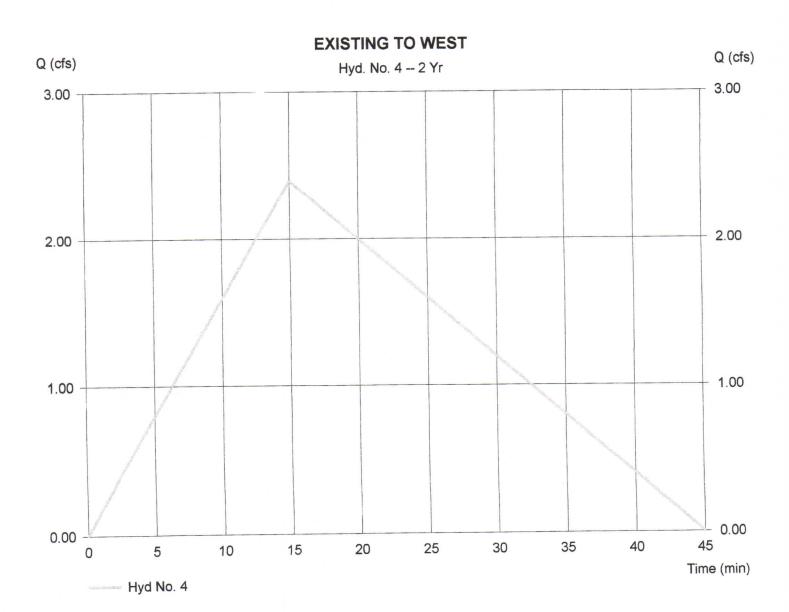
EXISTING TO WEST

Hydrograph type = Rational
Storm frequency = 2 yrs
Drainage area = 1.550 ac
Intensity = 3.031 in/hr
IDF Curve = rockland.IDF

Peak discharge = 2.40 cfs
Time interval = 1 min
Runoff coeff. = 0.51
Tc by User = 15.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 3,235 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

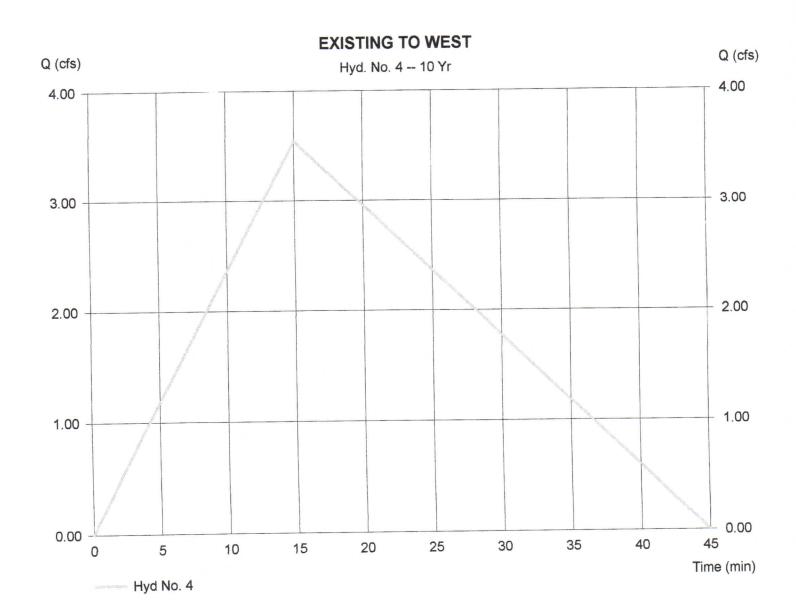
Hyd. No. 4

EXISTING TO WEST

Hydrograph type = Rational
Storm frequency = 10 yrs
Drainage area = 1.550 ac
Intensity = 4.491 in/hr
IDF Curve = rockland.IDF

Peak discharge = 3.55 cfs
Time interval = 1 min
Runoff coeff. = 0.51
Tc by User = 15.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 4,792 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

Hyd. No. 4

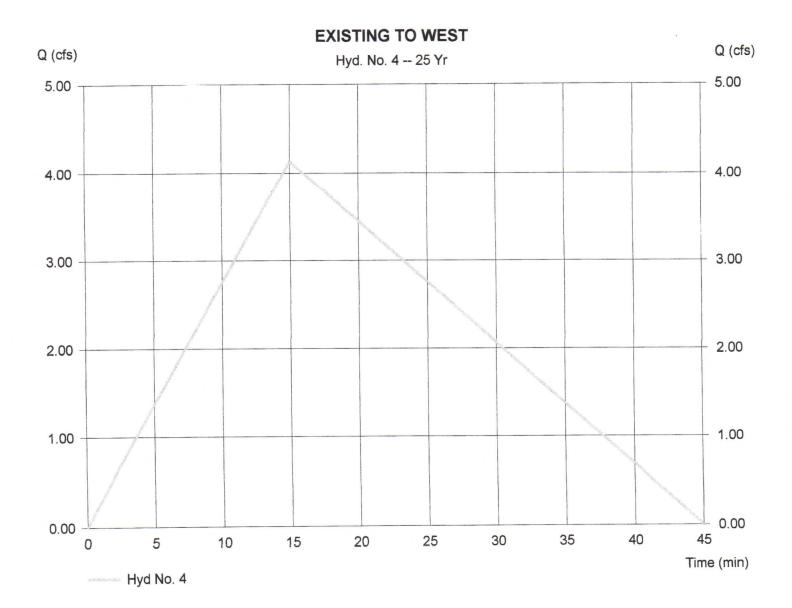
EXISTING TO WEST

Hydrograph type = Rational
Storm frequency = 25 yrs
Drainage area = 1.550 ac
Intensity = 5.228 in/hr
IDF Curve = rockland.IDF

Peak discharge = 4.13 cfs
Time interval = 1 min
Runoff coeff. = 0.51
Tc by User = 15.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 5,579 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

Hyd. No. 4

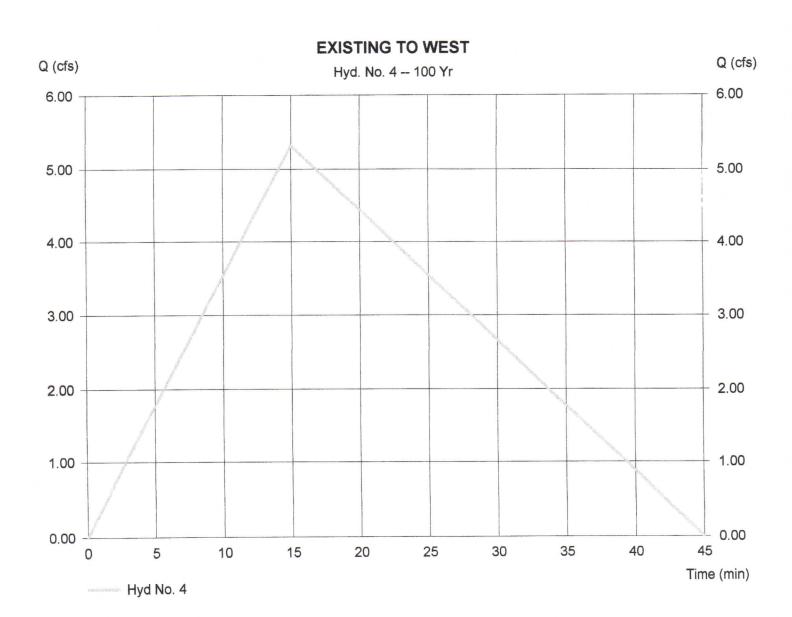
EXISTING TO WEST

Hydrograph type = Rational
Storm frequency = 100 yrs
Drainage area = 1.550 ac
Intensity = 6.730 in/hr
IDF Curve = rockland.IDF

Peak discharge = 5.32 cfs
Time interval = 1 min
Runoff coeff. = 0.51
Tc by User = 15.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 7,182 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

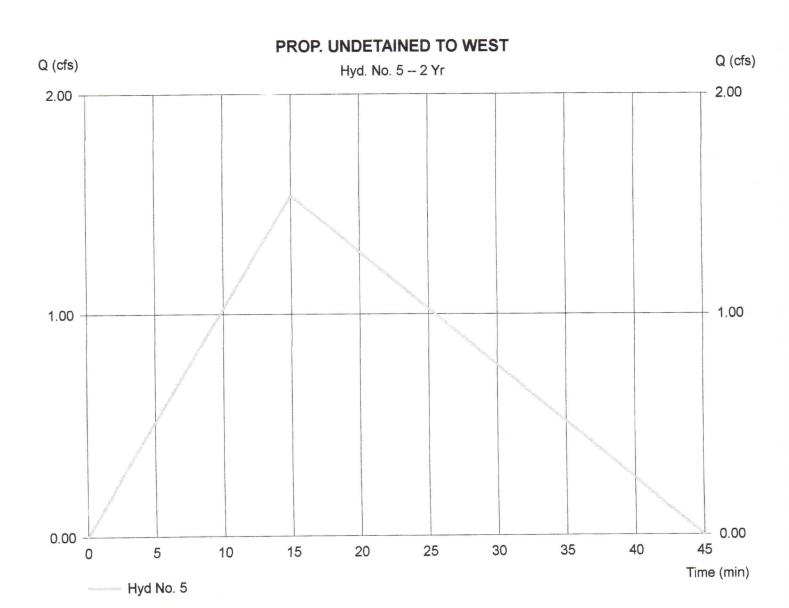
Hyd. No. 5

PROP. UNDETAINED TO WEST

Hydrograph type = Rational
Storm frequency = 2 yrs
Drainage area = 1.410 ac
Intensity = 3.031 in/hr
IDF Curve = rockland.IDF

Peak discharge = 1.54 cfs
Time interval = 1 min
Runoff coeff. = 0.36
Tc by User = 15.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 2,077 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

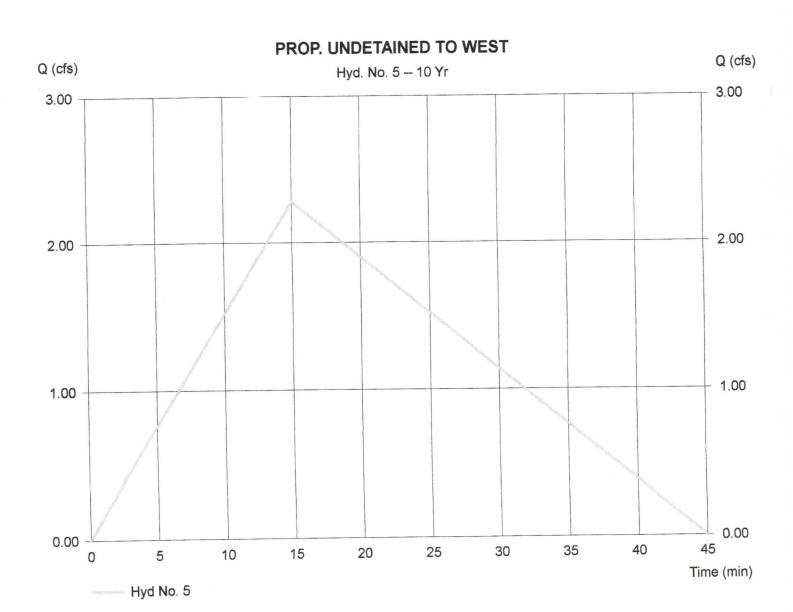
Hyd. No. 5

PROP. UNDETAINED TO WEST

Hydrograph type = Rational
Storm frequency = 10 yrs
Drainage area = 1.410 ac
Intensity = 4.491 in/hr
IDF Curve = rockland.IDF

Peak discharge = 2.28 cfs
Time interval = 1 min
Runoff coeff. = 0.36
Tc by User = 15.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 3,077 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

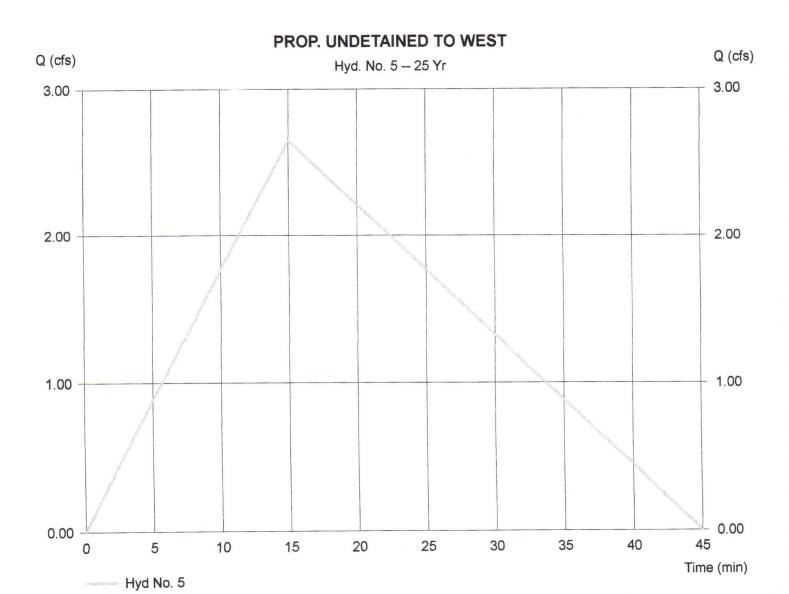
Hyd. No. 5

PROP. UNDETAINED TO WEST

Hydrograph type = Rational
Storm frequency = 25 yrs
Drainage area = 1.410 ac
Intensity = 5.228 in/hr
IDF Curve = rockland.IDF

Peak discharge = 2.65 cfs
Time interval = 1 min
Runoff coeff. = 0.36
Tc by User = 15.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 3,582 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

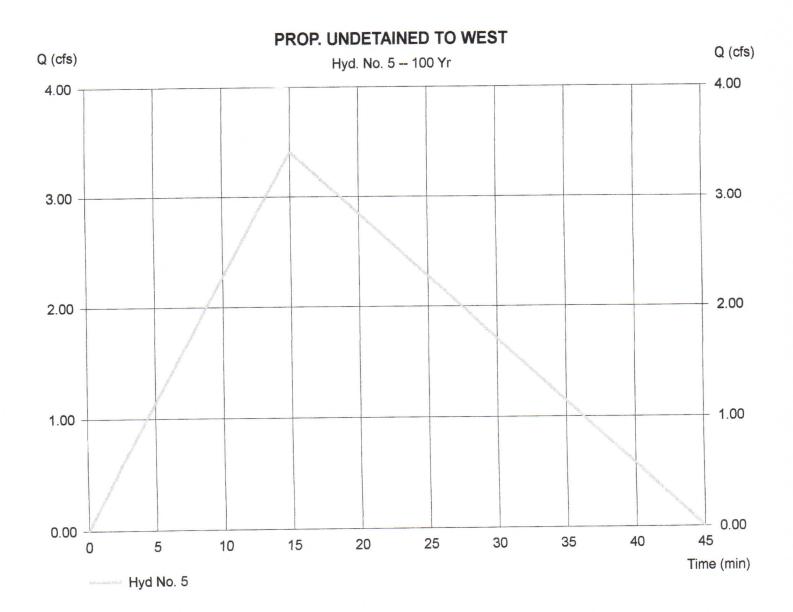
Hyd. No. 5

PROP. UNDETAINED TO WEST

Hydrograph type = Rational
Storm frequency = 100 yrs
Drainage area = 1.410 ac
Intensity = 6.730 in/hr
IDF Curve = rockland.IDF

Peak discharge = 3.42 cfs
Time interval = 1 min
Runoff coeff. = 0.36
Tc by User = 15.00 min
Asc/Rec limb fact = 1/2

Hydrograph Volume = 4,612 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

Hyd. No. 6

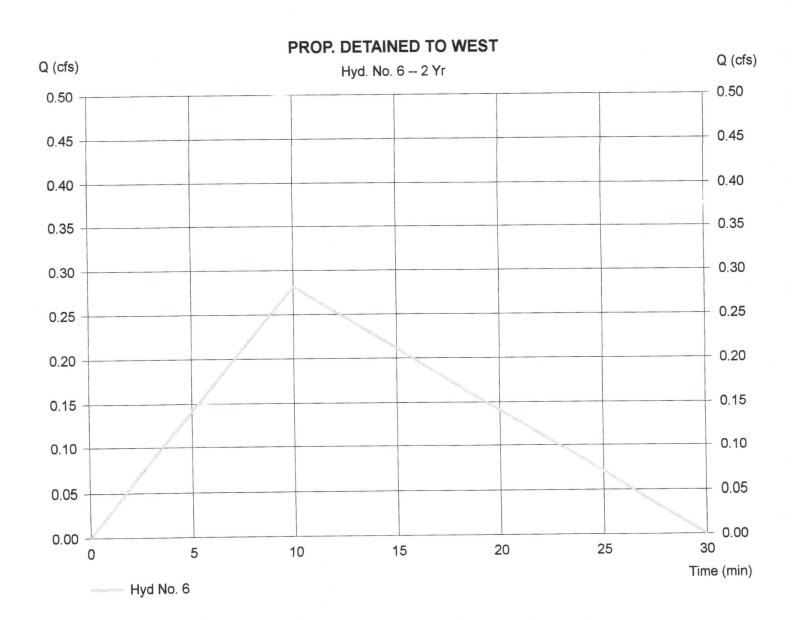
PROP. DETAINED TO WEST

Hydrograph type = Rational
Storm frequency = 2 yrs
Drainage area = 0.080 ac
Intensity = 3.556 in/hr
IDF Curve = rockland.IDF

Peak discharge = 0.28 cfs
Time interval = 1 min
Runoff coeff. = 0.99
Tc by User = 10.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 253 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

Hyd. No. 6

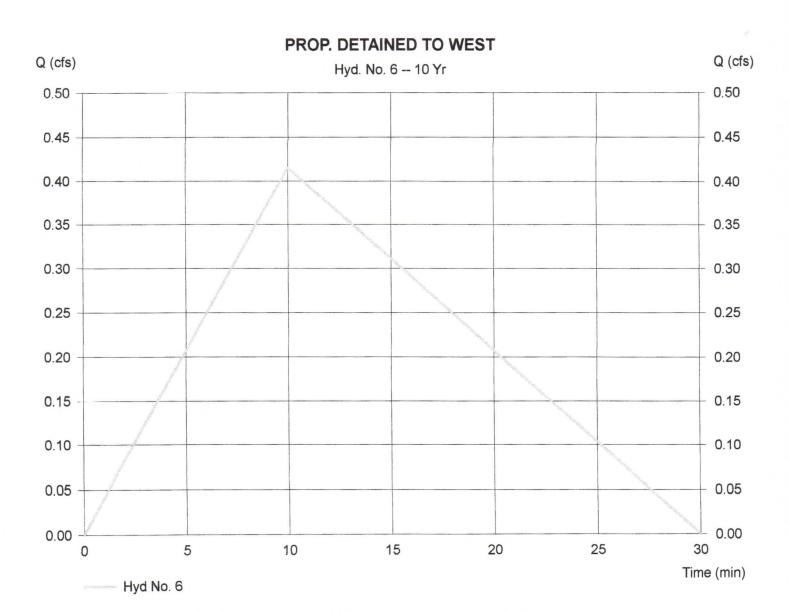
PROP. DETAINED TO WEST

Hydrograph type = Rational
Storm frequency = 10 yrs
Drainage area = 0.080 ac
Intensity = 5.240 in/hr
IDF Curve = rockland.IDF

Peak discharge = 0.41 cfs
Time interval = 1 min
Runoff coeff. = 0.99
Tc by User = 10.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 373 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

Hyd. No. 6

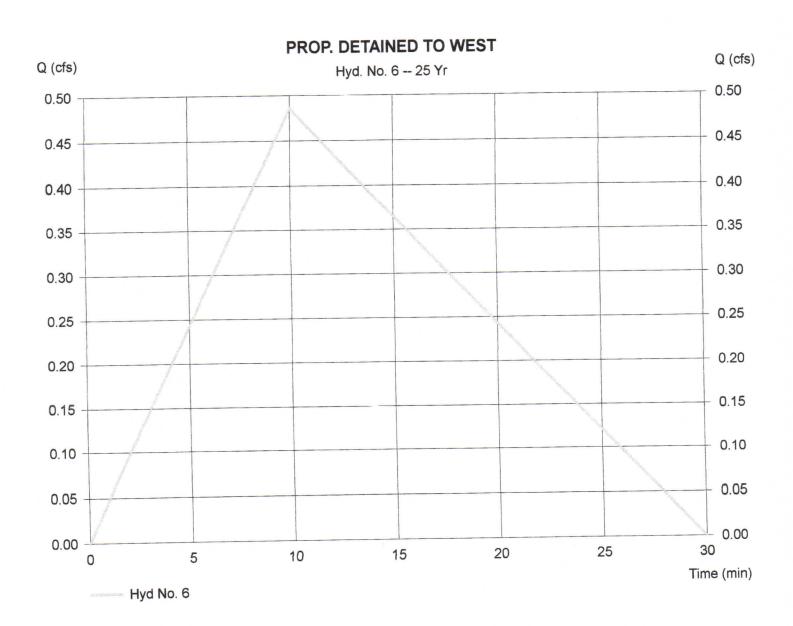
PROP. DETAINED TO WEST

Hydrograph type = Rational
Storm frequency = 25 yrs
Drainage area = 0.080 ac
Intensity = 6.135 in/hr
IDF Curve = rockland.IDF

Peak discharge = 0.49 cfs
Time interval = 1 min
Runoff coeff. = 0.99
Tc by User = 10.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 437 cuft



Hydraflow Hydrographs by Intelisolve

Monday, Jan 13 2014, 11:24 AM

Hyd. No. 6

PROP. DETAINED TO WEST

Hydrograph type = Rational
Storm frequency = 100 yrs
Drainage area = 0.080 ac
Intensity = 7.774 in/hr
IDF Curve = rockland.IDF

Peak discharge = 0.62 cfs
Time interval = 1 min
Runoff coeff. = 0.99
Tc by User = 10.00 min

Asc/Rec limb fact = 1/2

Hydrograph Volume = 554 cuft

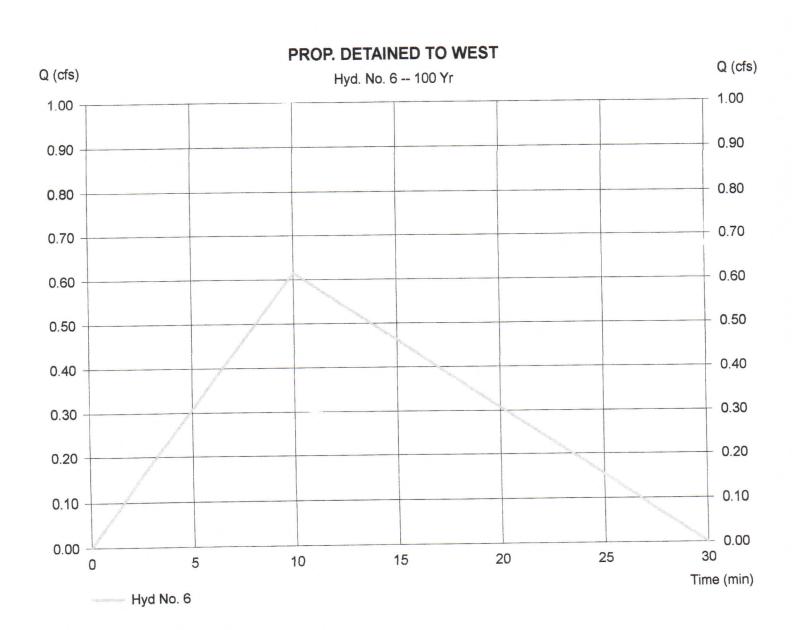


TABLE 3.1-2(B-1)
RUNOFF COEFFICIENTS
(AMC II)

	Hydro. Soil Group			
LAND USE DESCRIPTION			С	D
ultivated land :without conservation treatment	.49	.67	.81	.88
ultivated land :without conservation treatment :with conservation treatment	.27	.43	.61	.67
154540	.38	.63	.78	.84
Pasture or range land: poor condition good condition		.25	.51	.65
leadow: good condition	_ = =	ca ca co	. 44	.61
a mulch		.34	.59	.70
Wood or Forest land: thin stand, poor cover, no mulch good cover			.45	.59
Open Spaces, lawns, parks, golf courses, cemeteries				
good conditions: grass cover on 75% of more of	gas con 400	. 25	Section .	.65
the area fair condition: grass cover on 50% to 75% of				
fair condition: grass cover on 50% to 75% of the area		.45	.63	.74
Commercial and business areas (85% impervious)	.84		.93	.96
Industrial districts (72% impervious)	.67	.81	.88	.92
Residential:				
Average lot size Average % Impervious				
1/8 acre or less 65	.59			
1/4 acre 38	. 25	.55	.70	.80
1/3 acre 30		.49	.67	.78
1/2 acre 25		.45	.65	.76
1 acre 20		.41	.63	.74
Paved parking lots, roofs, driveways etc.	.99	.99	.99	.99
Streets and roads:				
paved with curbs and storm sewers	.99	0.00		
gravel	.57	.76		
dirt	.49	.69	.80	.84

NOTE: Values are based on S.C.S. definitions and are average values derived by an Advisory Committee for this Manual.

FIG. 3.1-2(D-1)-RAINFALL INTENSITY CURVES



